

### INTERNATIONAL SNOW SCIENCE WORKSHOP 2016 – BRECKENRIDGE



ctive works in a starting zone are interventions made A with more or less complex engineering structures

(usually snow nets, snow bridges, rakes, tripod stands,

etc.) designed and built to hold back the snow cover in the

releasing area by limiting the gravitational phenomena

Their secondary, but not less important function is to

create "breakages" (linear in case of a continuous positioning

positioning) within the potentially unstable slab so as to

reduce the destabilizing forces and, as a consequence, to

ACTIVE WORKS IN A STARTING ZONE

snow nets, snow bridges, rakes, tripod stands, etc. ...

Primary function

hold back the snow cover in the releasing area by limiting

the gravitational phenomena (snowflow) of the snowpack  $\stackrel{\bullet}{\Psi}$ 

Secondary function create "breakages" within the potentially unstable slab

In many cases, not only are the works or the single

handiwork exposed to the stresses related to their function

of snowpack retention in the starting zone, but they are also subject to "external" stresses which are not consistent

with the function for which those active works were

he on-site installation involves the construction of

continuous lines or, in some cases, interrupted lines of snow barrier structures able both to counteract the

snow creep phenomena and to interrupt the slab continuity.

The on-site placement, both in terms of distance between

the lines and in terms of coverage of the potential detachment area, must consider in detail the morphology of

the slope (in particular: the slope angle  $\Psi$ , the coefficient of

friction  $\boldsymbol{\phi}$  snow-ground, the height HK of the protection

works and the glide factor N) as well as the potential issues,

not directly linked to the natural accumulation of the

snowpack (e.g. snow brought by the wind) which can

ACTIVE WORKS IN A STARTING ZONE Stresses related to their function of snowpack retention in

the starting zone

Snow Creep Ψ

"External" stresses which are not consistent with the

function for which those active works were conceived.

designed and installed

Overload from windblown snow

Avalanche beetween the lines

Rock fall

Landslides and debris flow

SNOW CREEP

The solicitation of gliding in the line of slope generally depends on the slope angle  $[\Psi]$ , the snowpack density  $[\rho]$ ,

the height of the snow cover [H], the exposure and the

roughness of the contact plan between the snowpack [N]

The Swiss and French directives regulate the input loads to be applied to the calculation of the structures in terms of action related to snow gliding and express this stress

and the around.

through the following relations.

generate additional stresses on the protection structure.

punctual in case of an interrupted fragmentary

(snowflow) of the snowpack.

limit the avalanche trigger.

conceived, designed and installed.

or



# ACTIVE WORKS IN A STARTING ZONE: MONITORING MAINTENANCE AND FUNCTIONAL RESTORATION. AN ANALYSIS OF REAL CASES OF INTERVENTION

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UF A MWSL SNV Defence structures in avalanche starting zones

$$S'_N = \rho \cdot g \cdot \frac{H^{\cdot}}{2} K \cdot N \cdot f_c$$

where S'N

$$\rightarrow$$
 component o, ..... pressure in the line of slope

[kN/m] general aver ge den ity of snow  $[t/m^3]$ ρ  $\rightarrow$ 

- g  $\rightarrow$ gravitational acels ation [m/s<sup>2</sup>]
- H →
- general snow height [m]
- κ  $\rightarrow$ creep factor [-] N  $\rightarrow$
- glide factor 1 f<sub>c</sub>  $\rightarrow$

## height factor [-]

### AFNOR NF P 35-304

Equipements de protection contre les avalanches - Filet 

Table 1 summarizes the mintim of the load Fn orthogonal to the protection work. he :xt eme values vary from 9.90 kN/m to 75.2 kN/m as common of the glide factor N [-] and the height of the snownask  $\sqcup_n$  [m].

#### Tab. 1: total pressure (art 4 ° ~ NF P 95-30)

Ν	2 m	3 m	4 m	5 m
2,0	9,9		39,6	61,8
2,5	12,0	27 1	48,2	75,2

OVERLOAD FRC ... WINDBLOWN SNOW

The action of wind transport for the windward side to the leeward side generally use mines the creation of slabs along the leeward side. The phenomenon can create two main problems for active protection works: the first relating to the potential being of the work and the second concerning the increase f the 1 ad on the work. According to the empirical relation , one d by Föhn (1980), the value of win

$$H_{sd(1aa)} = k \cdot V^{\circ}$$
 for V  $\leq$  20 m/s

empiric ' 'ic:nt equal to 0,00008

v daily average v lues of wind speed [m/s]



accumulation

In the literature, the excreme values of wind loads for a 24-hour wind action can reach the values shown in Table 2.

Tab. 2: wind loads during 24 hours				
V [m/s]	Hs [cm/d]			
5 - 10	5 - 10			
10 - 15	10 - 35			
15 - 20	35 - 75			
20 - 25	75 - 200			
<b>&gt;</b> 25	<b>&gt;</b> 200			

AVALANCHE BEETWEEN THE LINES

PAPF

PIANIFICAZIONE, SVILUPPO E DIFESA DEL TERRITORIO

Special snowmaking conditions linked to high diurnal temperature increases and low nightly refreezing create favorable conditions for the humidification of the snowpack up to the basal layer and an increase in density.

These conditions, which generally occur earlier in the season or in the spring, allow the triggering of creeping phenomena and the formation of ground avalanches of humid and wet snow.



Fig. 2: example of ground avalanche between the lines

ROCK FALL

In many starting zones, rocky outcrops generate phenomena of collapse with rockfall events particularly in periods with higher temperature gradient between day and night.



Fig. 3: example of the impact of rock blocks on snow nets

#### LANDSLIDES AND DEBRIS FLOW

In starting zones with modest coverage debris (for example land class 4 type smooth scree mixed with earth), localized landslides or small debris flow are frequently seen especially in periods when snowpack is absent.

he stresses, in relation to the deformability of the structures, can cause alterations in the geometry or damage to the snow nets that may compromise their performance or functionality.

The stress due to snow creep generates standard load conditions both in rigid and in flexible snow barriers structures while the stresses caused by both distributed dynamic loads and concentrated dynamic loads can cause greater problems on rigid structures and semi-rigid structures. whereas the deformability of flexible structures allows areater stress distribution

### ACTIVE WORKS IN A STARTING ZONE

"EXTERNAL" STRESSES Ĵ

Problems at the foundation structures Problems at the structure

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nd accumulation is:  

$$H_{V} = k \cdot V^{\circ}$$
 for  $V \leq 20 \text{ m/s}$ 

where:

accumulation of snow brought by the wind  $H_{sd(1gg)} \rightarrow$ in 24 h ars [m/ 1]

[s<sup>3</sup>d<sup>-1</sup>m<sup>-2</sup>1



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PROBLEMS AT THE FOUNDATION STRUCTURES

The problems at the foundation of active works are mainly

caused by asymmetric load (e.g. wind overload) or punctual

dynamic stresses (e.g. rockfall) or distributed loads (e.g.

small avalanches between the lines, landslides or debris

flows).





PIANIFICAZIONE. SVILUPPO E DIFESA DEL TERRITORIO

# ACTIVE WORKS IN A STARTING ZONE: MONITORING MAINTENANCE AND FUNCTIONAL RESTORATION. AN ANALYSIS OF REAL CASES OF INTERVENTION

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PROBLEMS AT THE STRUCTURE

The issues to the structure of active works are primarily caused by the following concer ons: punctual dynamic load conditions (e.g. rockfall) or listributed dynamic load conditions (e.g. small avalan hes between the lines, landslides or debris flows



Fig. 4: example of a complete pulling out of an uphill anchor in a snow net



Fig. 5: example of a complete pulling out of an downhill anchor in a snow net



Fig. 6: example of the failure of the foundation plinth on the support of a snow bridge



Fig. 7: example of downhill rotation and translation of the post foundation of snow nets

Fig. 8: example of damag\_ ... ... structure of snow nets



Fig. 9: example of damages 1 the crossbeams in the structure of snov baildage

With reference to the iment of the physical condition of supporting structures shown in the Swiss directive, in Table 3 we summarize the degrees of maintenance according to the leed for intervention, the time of the problem onset and the consequences which could be triggered for inact on

Tab. 3: condition class				
Urgency	Loss of	Onset of the	Consequently	
	security	damage	the damage	
Condition class 1 «good»				
none	low	<ul> <li>5 years</li> </ul>	none	
Condition class 2 «damage				
1-3 years	medium	· -5 years	none	
Condition class 3 «poor»				
very	high	1 year	extreme impairment	

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increase of the urgency maintenance increase of the loss of security decrease of the time for the onset of the damage increase of the consequently the damage

At the same time, as the degree increases, the time period for the occurrence of possible damage decreases, as schematically shown in Table 4.

Tab. 4: possible damage		
Condition class	M	ain type of damage
Condition class 1 (good»	<ul> <li>defor</li> <li>erosia</li> <li>10-20</li> <li>collection</li> <li>thicket</li> <li>unifor</li> </ul>	med crossbeams on of foundation block < ) cm ttion of debris on grate ness < 50 cm rm surface corrosion
Condition class 2 «damage»	<ul> <li>slight</li> <li>displa</li> <li>micro</li> <li>the gi</li> <li>exposision (still)</li> </ul>	ly deformed supports iced cable clips pile anchors pushed into round ied anchors > 20-40 cm intact)
Condition class 3 (poor»	<ul> <li>buckli</li> <li>heavil</li> <li>girder</li> <li>broke</li> <li>buckli</li> <li>broke</li> </ul>	ed supports ly deformed or broken r en or pulled out anchors ed micropiles en wire ropes

INCREASE OF THE CLASS CONDITION

increase of damage increase of maintenance

Table 5 shows, according to the degree of maintenance, the type of intervention and the time of achievement.

Tad. 5: sequence of maintenance				
Condition class	Types of interventions			
Condition class 1 «good»	inspection and checking after every major event or every 3- 5 years			
Condition class 2 «damage»	repair wthin 1-3 years			
Condition class 3 «poor»	immediate repairs or replacement before the winter			

INCREASE OF THE CLASS CONDITION

increase of maintenance decrease of the time for the maintenance

onclusion ... The maintenance of active works in the Crelease area, regardless of the type of work, involves specific issues connected with constructions sites and logistics, due to the particular location of the works.

To this purpose, a preliminary monitoring is a fundamental step to assess the time of intervention and to define maintenance and repair activities.

In parallel to maintenance, a number of preliminary activities will have to be considered in order to improve the areas of intervention so as to face potential critical environmental issues (such as rock falls, landslides, etc. ...).

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